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For the long- term condition, the sum of the factored loads is 1x300 +1.25x94 = 417 kN and the factored resistance is 0.77x543 = 418 kN. According to the Eurocode, therefore, the long-term condition is acceptable.

LOAD (kN) - Fellenius

Phase 1 comprised four load increments applied every one hour up to the desired working load (6,154 kN), which was held for six hours, whereupon four additional load increments were applied to twice the working load, which was held for 36 hours.

LOAD, Q (kN) - Fellenius

Load Kn Fellenius Phase 1 comprised four load increments applied every one hour up to the desired working load (6,154 kN), which was held for six hours, whereupon four additional load increments were applied to twice the working load, which was held for 36 hours. LOAD, Q (kN) - Fellenius For the long- term condition, the sum of the factored loads is

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The loadLOAD (kN) was applied by hydraulic jacks working against a loaded platform 2 and were measured by a separate load cell. The pile head movement was measured by dial gages acting against a reference beam.

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LOAD (KN) DEPTH (m) Fellenius et al. 2004. Static Loading Test. at Pend Oreille, Sandpoint, Idaho, for the realignment of US95 . 406 m diameter, 45 m embedment, closed-toe pipe pile driven in soft clay. Clay. 200+ m

Views on Accuracy of Tests and Analyses - fellenius.net

The remediation work included construction of twenty-eight, 1,300 mm diameter, about 30 m long, bored piles, each with a working load of almost

9,000 KN (2,000 kips), founded in a sand and gravel deposit. It was imperative that all construction work was finished in six months (September 1994, the start of the football season).

The static loading test bengt h. fellenius
The maximum drag load is about 600 KN. Thus, adding the 700 KN dead load, the maximum axial load will be about 1,300 KN, which is well within the structural strength of the pile. The bridge pier will be placed on 15 piles and the footprint of the pile cap is 1.5 m times 15 m.

Fellenius, B.H. and Goudreault, P.A, 2013. Two case ...
As is visually presented in Fig. 1, applying a factor of safety of 2.0 results in an allowable load of 700 KN (dead load 600 KN and live load 100 KN). Moreover, due to the fill and a lowering of the groundwater table, an almost 200 mm settlement of the ground surface will develop after the construction.

Y Bengt H. Fellenius DRAG A FORCE L
Fellenius Amsterdam May 27, 2016. www.Fellenius.net. 4 ... LOAD (KN) STATIC TEST DYNAMIC TEST 0 500 1,000 1,500 2,000 0 100 200 300 MOVEMENT (mm) LOAD (KN) DYNAMIC TEST 0 500 1,000 1,500 2,000 0 100 200 300 MOVEMENT (mm) LOAD (KN) DYNAMIC TEST in a series of blows Repeated

BengtH. Fellenius - KIVI
Fellenius, B. H., 2001. From strain measurements to load in an instrumented pile. Geotechnical News Magazine, Vol. 19, No. 1, pp 35 - 38. From Strain Measurements to Load in an Instrumented Pile Bengt H. Fellenius Introduction More and more, our profession is realizing that a conventional static loading test on a pile provides limited information.

From Strain Measurements to Load in an Instrumented Pile
8.3 Hansen Failure Load 8.4 Chin-Kondner Extrapolation 8.5 Decourt Extrapolation 8.6 De Beer Yield Load 8.7 The Creep Method 8.8 Load at Maximum Curvature 8.9 Factor of Safety 8.10 Choice of Criterion 8.11 Loading Test Simulation 8.12 Determining Toe Movement 8.13 Effect of Residual load 8.14 Instrumented Tests

Basics of Foundation Design - UniFI
The load increments were 50-kN, each with a 10-minute load-holding time. At the maximum load of 700 kN, the lower length (1.6 m) plunged. No movement occurred at the pile head and the net BDC opening was 60+ mm. 16.4m 13.8m 9.3m 1.8m Strain Gages Strain Gages Ground Surface PILE Strain Gages BDC

Static load testing and prediction bengt h. fellenius
Load distributions have also been calculated from the SPT-indices using the Decourt, Meyerhof, and O'Neil-Reese methods, as well that from the Eslami-Fellenius CPTU-method. 36 0 5 10 15 20 25 0 500 1,000 1,500 2,000 DEPTH(m) LOAD (kN) SPT-Meyerhof Test 0 5 10 15 20 25 0 10 20 30 40 50 DEPTH(m) N (blows/0.3m) compact SAND CLAY compact SAND ...

Unisoft software bengt h. fellenius, pierre goudrault
Fellenius (1988) identified changes in effective stress during reconsolidation of the soil after pile construction as a possible cause of negative skin friction development.Fellenius (1969), Leung at al. (1991), and Leifer (1992) showed that a transient (live) load will not coexist with a

NCHRP12-116 PROPOSED AASHTO SPECIFICATIONS FOR DESIGN OF ...

The load distribution of the uncoated pile shows how the negative skin friction accumulates to a maximum drag load value of about 1,100 KN at about 25 m depth. Below this depth, the shaft shear turned to positive shaft resistance and the load in the pile reduced with depth to a mobilized toe resistance of about 550 KN.

Results from long-term measurement in piles of drag load ...

The residual load at the pile toe was 42·1 kN, which was 60·0% of the toe load. The residual load in the test pile was mostly due to the rebound of soil below the pile toe and the recovery of soil strength after soil disturbance due to pile installation (Fellenius, 2002). 3.2 Load–displacement response during the pile load test

Pile load test of jacked open-ended prestressed high ...

As shown in fig. D2 in Fellenius (2015), there is misleading information. When the analysis was rerun, the increase of axial load for outer pile TP-2 was extrapolated, and is estimated to be 120 kN not 80 kN. Figure R1 shows the load–movement curves resulting from applying the data from fig. 12 in Ko and Jeong (2015). The measured data of the inner pile at the first gauge level is not 610 kN, but is 814 kN due to an error that is illustrated in fig. D2 by the discusser.